

A3.4 SUBROUTINE LISTING

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SUBROUTINE EL2D14 (A-H,O-Z)
IMPLICIT REAL*8 (A-H,O-Z)
COMMON /EL/ IND, ICONT, NPAR(20), NUMEG, NEGL, NEGHL, IMASS, IDAMP, ISTAT
1 COMMON /DIMEL/ N101, N102, N103, N104, N105, N106, N107, N108, N109, N110,
1 N111, N112, N113, N114, N120, N121, N122, N123, N124, N125
COMMON /MATMOD/ STRESS(4), STRAIN(4), D(4,4), IPT, HEL, IPS
COMMON /DPR/ ITWO
COMMON A(1)
REAL A
DIMENSION IA(1)
EQUIVALENCE (NPAR(10), NINT), (A(1), IA(1))
IDW=11*ITWO
NPT=NINT*NINT
MATP=IA(N107+NEL-1)
NM=N109+(MATP-1)*17*ITWO
IP (IND.NE.0) GO TO 100
1 INITIALIZE WORKING SPACE ARRAY WA 1
NM=N110+(NEL-1)*NPT*IDW
CALL EPLA (A(NM), A(NM), A(NH), NPT, IDW)
GO TO 599
1 FIND STRESS STRAIN LAW AND STRESSES 1
NM=N110+(NEL-1)*NPT*IDW+(IPT-1)*IDW
CALL EPLA (A(NM), A(NM), A(NH+4*ITWO), A(NH+8*ITWO), A(NH+9*ITWO),
1 A(NH+10*ITWO))
CONTINUE
RETURN
END
SUBROUTINE EPLA (WA, IWA, PROP, NPT, IDW)
IMPLICIT REAL*8 (A-H,O-Z)
DIMENSION WA(1), IWA(IDW,1), PROP(2)
COMMON /DPR/ ITWO
1 SET INITIAL STRESSES AND STRAINS TO ZERO
1 SET INITIAL STRESS STATE TO ELASTIC.
DO 10 J=1, NPT
DO 15 I=1, 8
WA(I,J)=0.0
WA(9,J)=PROP(9)
KJ=10*ITWO+1
IWA(KJ,J)=1
RETURN
END

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YCX00010
YCY00020
YYCY00030
YCA00040
YCX00050
YCY00060
YCX00070
YCX00080
YCY00090
YCY00100
YCX00110
YCY00120
YCY00130
YCY00140
YCX00150
YCY00160
YCX00170
YCX00180
YCY00190
YCY00200
YCX00210
YCY00220
YCY00230
YCX00240
YCY00250
YCY00260
YCX00270
YCX00280
YCY00290
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SUBROUTINE EPLAL (PROP, SIG, EPS, XK, XXALPA, IPEL)  
 .....  
 "THIS SUBROUTINE CALCULATES THE STRESSES AND STRESS STRAIN  
 LAW OF AN ISOTROPIC HARDENING MATERIAL SUBJECT TO THE  
 DRUCKER-PRAGER YIELD SURFACE, IN PLANE STRESS CONDITIONS  
 .....  
 THE FOLLOWING VARIABLES ARE USED IN THIS SUBROUTINE  
 .....  
 ISTR NUMBER OF STRESS COMPONENTS  
 SIG STRESSES AT THE END OF THE PREVIOUS UPDATE  
 EPS STRAINS AT THE END OF THE PREVIOUS UPDATE  
 RATIO PART OF STRAIN INCREMENT TAKEN ELASTICALLY  
 DELSIG INCREMENT IN STRAINS  
 DELSIG INCREMENT IN STRESSES  
 .....  
 PROP (1) MODEL TYPE (V. HISES-D. PRAGER)  
 PROP (2) PLASTIC POTENTIAL (ASS. -NON ASS.)  
 PROP (3) LINEAR STRAIN HARDENING GRADIENT  
 PROP (4) LITHNIT RIAL CONSTANT "AU"  
 PROP (5) EXPONENTIAL CONSTANT "BU"  
 PROP (6) YOUNG'S MODULUS  
 PROP (7) POISSON'S RATIO  
 PROP (8) KAPPA - YIELD SURFACE PARAMETER  
 PROP (9) ALPHA - YIELD SURFACE PARAMETER  
 PROP (10) STRAIN HARDENING MODULUS  
 PROP (11) KCUTT-FAILURE SURFACE PARAMETER  
 PROP (12) ALCUTT-FAILURE SURFACE PARAMETER  
 PROP (13) CY YIELD SURFACE PARAMETER C  
 PROP (14) DIFFERENCE BETWEEN YIELD-FAILURE PARAMETERS  
 PROP (15) KAPPA  
 PROP (16) DIFFERENCE BETWEEN YIELD-FAILURE PARAMETERS  
 .....  
 IPEL = 1, MATERIAL ELASTIC  
 = 2, MATERIAL PLASTIC  
 = 3, FAILURE-STIFFNESS REDUCTION, PLASTIC  
 = 4, FAILURE-STIFFNESS REDUCTION, PLASTIC  
 .....  
 NOTE: CERTAIN STATEMENTS ARE REDUNDANT IN TERMS OF THE  
 PROGRAMS PRESENT FORM; FURTHER MODIFICATION IS RE-  
 QUIRED TO INCLUDE: AXISYMETRIC  
 TOTAL & UPDATED LAGRANGIAN NON-  
 LINEAR FORMULATIONS

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1  IMPLICIT REAL*8 (A-H,O-Z)
COMMON /EL/ INEB,ACOUNT,MPAR(20),XUNEG,NEGL,NEGHI,INASS,IDAHP,ISTAT
1  COMMON /VAR/ HCF,HC,HODPEY,IPAL,CI,B1,CT,D1,B2,BH,CEE,DEPS(4),TEPS(4)
1  COMMON /VHISES/ A1,B1,CT,D1,B2,BH,CEE,DEPS(4),TEPS(4)
1  COMMON /DRUK/ UTT,YKAP,ALC,YF,YC,DELTA(3),IN,SHL,EXPA,EXPB,
1  ALFBA,BET
COMMON /MATHOD/ STRESS(4),STRAIN(4),D(4,4),IPT,NEL,IPS
COMMON /DISDR/ DISD(5)
COMMON /TODIN/ BETA,THIC,DEIEL,NMDS
1  DIMENSION DEEPS(4),STATE(5)
EQUIVALENCE (MPAR(5),INDEL), (MPAR(5),ITYP2D), (MPAR(15),MODEL)
INTEGER PROP(1),THN
DATA STATE /2H F,2HFR,2HFE,2HFE/, NGLAST /1000/
XKAP=YKAP
ALFPA=XXALFPA
IPELDS=IPEL
STRESS(4)=0.
DELEPS(4)=0.
THN=PROP(1)
PPOT=PROP(2)
SHL=PROP(3)
RYLIM=PROP(4)
EXPA=PROP(5)
EXPB=PROP(6)
PV=PROP(7)
YKAP=PROP(8)
ALFPA=PROP(9)
ET=PROP(10)
UTT=PROP(11)
ALC=PROP(12)
YF=PROP(13)
YF=PROP(14)
YF=PROP(15)
IF (ST.EQ.0.) ST=1.
IP (UTT.EQ.0.) UTT=1000.*YK
1  ISTAT=3
1  ISR=3
1  CEE=1.-PV*PV
1  B2=(1.-PV)/2
..... PLANE STRESS .....
..... A1=YM/CEE .....
..... B1=A1*PV .....
..... D1=PV/(PV-1.) .....

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VCX02160
VCX02170
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VCX02190
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GO TO 622
... STATE OF STRESS OUTSIDE LOADING SURFACE - PLASTIC BEHAVIOUR ...
... DETERMINE THE PORTION OF STRAIN TAKEN ELASTICALLY ...
...
IPEL=2
IF (IPEL.EQ.3) IPEL=0
IF (IPEL.EQ.3) AND.RATIO.EQ.0.) GO TO 378
IF (IPEL.EQ.0) IPEL=3
IF (IPEL.EQ.2) AND.RATIO.EQ.0.) GO TO 303
IF (IPEL.EQ.3) GO TO 315
IF (IPEL.EQ.2) *(DM**2) #9
BR=RB+ (6*XKD*ALPHA*DH) - (18*ALPHA*ALPHA*SH*DH)
DR=RD+ (6*XKD*ALPHA*SH) - (9*ALPHA*ALPHA*SH*SH)
GO TO 316
AR=RA - (ALC**2) *(DH**2) #9
BR=RB+6*UTT*ALC*DH-18*ALC*ALC*SH*DH
DR=RD+6*UTT*ALC*SH - (UTT**2) -9*(ALC**2) *(SN**2)
R= (BR**2) -4.*AR*DR
IF (R.LT.0.) GO TO 302
RZ= (-BR+DSORT(R)) / (2.*AR)
RV= (-BR-DSORT(R)) / (2.*AR)
IF (RZ.GE.0. AND.RZ.LE.1.) RATIO=RZ
IF (RV.GE.0. AND.RV.LE.1.) RATIO=RV
RATIO=RZ
IF (RATIO.LT.0.) GO TO 302
IF (RATIO.LT.1E-03) RATIO=0.
GO TO 303
IF (RZ.LT.0.) GO TO 304
RATIO=RZ
IF (RV.LT.0.) RATIO=RV
IF (RATIO.LE.1E-03) RATIO=0.
GO TO 303
RATIO=RV
IF (RATIO.LE.1E-03) RATIO=0.
WRITE (6,2200)
STOP
IF (IPEL.EQ.3) AND.RATIO.GT.0.) GO TO 317
IF (IPEL.EQ.3) AND.RATIO.EQ.0.) GO TO 378
DO 320 I=1,IST
STRESS(I)=SIG(I) +RATIO*DELSIG(I)
STRAIN(I)=EPS(I) +RATIO*DELEPS(I)+DELEPS(1)+DELEPS(2)
GO TO 378
... DETERMINE THE INCREMENT INTERVAL ...
INTER=25
IPEL=2
DO 312 I=1,IST
STRESS(I)=SIG(I)

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C 316
C 301
C 304
C 302
C 303
C 320
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C 317
C 312

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VCX027720
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VCX027740
VCX027750
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VCX030000
VCX030010
VCX030020
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378 STRAIN(4) = EPS(4)
      XM = RATIO / DFL0AT (INTER)
      GO TO 318
      INTER = 25
      IF (IPELD.EQ.3) IPELD=3
      IF (IPELD.NE.3) GO TO 313
      IF (RATIO.NE.0.) GO TO 313
      DO 314 I=1,IST
      STRESS(I) = SIG(I)
      STRAIN(4) = EPS(4)
      XM = (1 - RATIO) / DFL0AT (INTER)
      DO 380 I=1,ISR
      DEPS(I) = XM * DELEPS(I)
      ** CALCULATION OF ELASTO-PLASTIC STRESSES **
      ..
      DO 500 IN=1,INTER
      IP (IN.GT.1) GO TO 419
      DELP(1) = A1 * DEPS(1) + B1 * DEPS(2)
      DELP(2) = C1 * DEPS(1) + A1 * DEPS(2)
      CALL HEDLP
      DO 420 I=1,IST
      DO 420 J=1,ISR
      STRESS(I) = STRESS(I) + D(I,J) * DEPS(J)
      DELP(I) = DELP(I) + D(I,J) * DEPS(J)
      SM = (STRESS(1) + STRESS(2) + STRESS(4)) / 3.
      SX = STRESS(1) - SM
      SY = STRESS(2) - SM
      SZ = STRESS(4) - SM
      CONTINUE
      FTB = 3 * ALC * SH + (DSORT(.5 * (SX**2 + SY**2 + SZ**2) + STRESS(3)**2))
      FTB = FTB - UT
      IF (IPELD.EQ.3) AND.IPELD.EQ.3) GO TO 502
      IF (FTB.LE.0.) GO TO 502
      IPPL=1
      IPELD=3
      DO 501 I=1,IST
      DELSIG(I) = STRESS(I) - SIG(I)
      GO TO 510
      IPEL = IPELD
      IF (INDNL.NE.3) GO TO 600
      OMEGA = DISD(3) - DISD(4)
      STRESS(1) = STRESS(1) + OMEGA * SIG(3)
      STRESS(2) = STRESS(2) - OMEGA * SIG(3)
      STRESS(3) = STRESS(3) + 5 * OMEGA * (SIG(2) - SIG(1))
      ** UPDATE KAPPA (KK) **
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YCX03800

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IF (IPELD.EQ.3) GO TO 622
IF (TMM.EQ.1) GO TO 622
RD = (5*(SY**2+SZ**2)+STRESS(3)**2)
RT = (YF*(YKAP-DSQRT(RD))+XF*(YC-SH)}{YC-DSQRT(RD)}
RTT = (RT**2) - 4*XF*YF*(YKAP*(YC-SH) - YC*DSQRT(RD))
IF (RTT.LT.0.) RTT=0.
IF (RTT.EQ.0.) GO TO 622
RT = (DSQRT(RTT) - RT) / (2*XF*YF)
IF (RT.GT.1.) GO TO 622
XKDD=YKAP+RT*XF
IF (XKDD.LT.XK) GO TO 622
XKDD=XKDD
YALFA = (YKD / (YC+RT*YF)) / 3.
GO TO 622
XALFA=0.
XKDD=FTB+UTT
IF (XKDD.LT.XK) GO TO 622
XKD=XKDD

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623  
625

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...UPDATING STRESSES, STRAINS, KAPPA, IPEL...
...
IF (IUPDT.NE.0) GO TO 621
XK=XKD
XALFA=XALFA
IPEL=IPEL
DO 610 I=1,IST
SIG(I)=STRESS(I)
EPS(I)=STRAIN(I)
IF (KPRI.EQ.0) GO TO 700
IF (ICOUNT.EQ.3) RETURN

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...FORM THE MATERIAL LAW
...IN DIVERGENCE REFORMATION (IEOREF=1)
...ASSUME ELASTIC BEHAVIOUR
...
IF (IEOREF.EQ.1) GO TO 623
IF (IPELD.EQ.2) GO TO 650
IF (IPELD.EQ.3) GO TO 650
DO 625 I=1,4
DO 625 J=1,4
D(1,1)=A1
D(2,1)=B1
D(1,2)=A1
D(2,2)=B1
D(3,3)=C1
D(4,4)=0.

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D(4,2)=0.
D(4,3)=0.
D(4,4)=0.
IF(IPELD.EQ.1) RETURN
RETURN
CALL MEDIP
RETURN
650
C
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700
... PRINTING OF STRESSES ...
*****
DM=(STRESS(1)+STRESS(2)+STRESS(4))/3.
DY=STRESS(1)-DM
DZ=STRESS(2)-DM
DS=STRESS(3)-DM
PTA=.5*(DX**2+DY**2+DZ**2)+DS**2
FT=3*ALPHA*DM+DSQRT(PTA)-YKAP
IF (INDNL.NE.2) GO TO 800
CALL CAUCHY
IF (IPRI.NE.0) RETURN
IF (INDNL.LE.2) OR (ITYP2D.LT.2) GO TO 801
XBAR=THIC*DEXP(STRAIN(4))
IF (NG.NE.HGLAST) GO TO 802
IF (NEL.GT.HGLAST) GO TO 806
IF (IPT-1) 809,808,809
HGLAST=NG
IF (INDNL.EQ.3) GO TO 804
WRITE(6,2002)
GO TO 806
WRITE(6,2001)
HELAST=HEL
WRITE(6,2004) NEL
IF (IPELD.NE.3) GO TO 810
WRITE(6,2221)
CALL MAXMIN (STRESS,SX,SY,SM)
IF (INDNL.EQ.3) GO TO 814,813
WRITE(6,2005) IPT,STATE(IPELD), (STRESS(I),I=1,IST),SX,SY,SM,FT
GO TO 821
WRITE(6,2007) IPT,STATE(IPELD), (STRESS(I),I=1,3),SX,SY,SM,FT,XBAR
WRITE(6,2006) (EPS(I),I=1,3),EPS(4)
RETURN
1) THE MATERIAL HAS FAILED IN THIS REGION, STIFFNESS REDUCED
2) STRESS-YZ
3) STRESS-YX
4) STRESS-ZZ
5) STRESS-ZX
6) STRESS-XY
7) STRESS-YY
8) STRESS-ZY
9) STRESS-XX
10) EQUIVALENT STATE, 78X, 5HANGLE, 10X, 6HSTRESS, 5X,
16H NUM/IPT
9) THICKNESS
1) STRESS
2) MIN STRESS
3) MAX STRESS
4) MIN STRESS
5) MAX STRESS
6) STATE, 78X, 5HANGLE, 10X, 6HVALUE /
1) A2, 6HGLASTIC, 1X, 3E14.6, 3X, P6.2, 3X, E14.6,
2) STRAINS, T20, ST-YX, E14.7, ST-YZ,
3) ST-ZZ, E14.7, ST-ZX, E14.6,
4) ST-XY, E14.6, ST-YY, E14.6,
5) ST-ZY, E14.6, ST-XX, E14.6,
6) ST-YY, E14.6, ST-ZX, E14.6,
7) ST-ZY, E14.6, ST-XX, E14.6,
8) ST-XY, E14.6, ST-ZX, E14.6,
9) ST-ZY, E14.6, ST-XX, E14.6,
10) ST-XY, E14.6, ST-ZX, E14.6,
11) ST-ZY, E14.6, ST-XX, E14.6,
12) ST-XY, E14.6, ST-ZX, E14.6,
13) ST-ZY, E14.6, ST-XX, E14.6,
14) ST-XY, E14.6, ST-ZX, E14.6,
15) ST-ZY, E14.6, ST-XX, E14.6,
16) ST-XY, E14.6, ST-ZX, E14.6,
17) ST-ZY, E14.6, ST-XX, E14.6,
18) ST-XY, E14.6, ST-ZX, E14.6,
19) ST-ZY, E14.6, ST-XX, E14.6,
20) ST-XY, E14.6, ST-ZX, E14.6,
21) ST-ZY, E14.6, ST-XX, E14.6,
22) ST-XY, E14.6, ST-ZX, E14.6,
23) ST-ZY, E14.6, ST-XX, E14.6,
24) ST-XY, E14.6, ST-ZX, E14.6,
25) ST-ZY, E14.6, ST-XX, E14.6,
26) ST-XY, E14.6, ST-ZX, E14.6,
27) ST-ZY, E14.6, ST-XX, E14.6,
28) ST-XY, E14.6, ST-ZX, E14.6,
29) ST-ZY, E14.6, ST-XX, E14.6,
30) ST-XY, E14.6, ST-ZX, E14.6,
31) ST-ZY, E14.6, ST-XX, E14.6,
32) ST-XY, E14.6, ST-ZX, E14.6,
33) ST-ZY, E14.6, ST-XX, E14.6,
34) ST-XY, E14.6, ST-ZX, E14.6,
35) ST-ZY, E14.6, ST-XX, E14.6,
36) ST-XY, E14.6, ST-ZX, E14.6,
37) ST-ZY, E14.6, ST-XX, E14.6,
38) ST-XY, E14.6, ST-ZX, E14.6,
39) ST-ZY, E14.6, ST-XX, E14.6,
40) ST-XY, E14.6, ST-ZX, E14.6,
41) ST-ZY, E14.6, ST-XX, E14.6,
42) ST-XY, E14.6, ST-ZX, E14.6,
43) ST-ZY, E14.6, ST-XX, E14.6,
44) ST-XY, E14.6, ST-ZX, E14.6,
45) ST-ZY, E14.6, ST-XX, E14.6,
46) ST-XY, E14.6, ST-ZX, E14.6,
47) ST-ZY, E14.6, ST-XX, E14.6,
48) ST-XY, E14.6, ST-ZX, E14.6,
49) ST-ZY, E14.6, ST-XX, E14.6,
50) ST-XY, E14.6, ST-ZX, E14.6,
51) ST-ZY, E14.6, ST-XX, E14.6,
52) ST-XY, E14.6, ST-ZX, E14.6,
53) ST-ZY, E14.6, ST-XX, E14.6,
54) ST-XY, E14.6, ST-ZX, E14.6,
55) ST-ZY, E14.6, ST-XX, E14.6,
56) ST-XY, E14.6, ST-ZX, E14.6,
57) ST-ZY, E14.6, ST-XX, E14.6,
58) ST-XY, E14.6, ST-ZX, E14.6,
59) ST-ZY, E14.6, ST-XX, E14.6,
60) ST-XY, E14.6, ST-ZX, E14.6,
61) ST-ZY, E14.6, ST-XX, E14.6,
62) ST-XY, E14.6, ST-ZX, E14.6,
63) ST-ZY, E14.6, ST-XX, E14.6,

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YCX03810
YCX03820
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YCX04330
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YCX04350
YCX04360
YCX04370
YCX04380

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1E14.7,ST-XX,E14.7/
FORMA4(5X,I2,16,A2,7H,ELASTIC 3E14.6,3X,2E14.6,3X,F6.2,3X,2E14.6)
FORMAT(T10,'INCORRECT VALUE FOR RATIO,STOP',3E14.7)
END
C
SUBROUTINE MEDIP
IMPLICIT REAL*8 (A-H,O-Z)
COMMON /EL/ IND,ICOUNT,NPAR(20),NUMEG,NEGL,NEGHL,IMASS,IDAMP,ISTAT
COMMON /KLN/ IEIG,INASSH,IDAMPN
COMMON /VHSES/ A1,B1,C1,D1,B2,BK,CES,DEPS(4),EPS(4),TMM,PPOT
1 ISR,IST,ALFA,PV,ET,ST,FTB,IPELD,IPPL,RLIM,TMM,PPOT
COMMON /DROUK/ DTT,YKAP,ALC,YP,XP,YC,DELP(3),YM,SHL,EXPA,EXPB,
1 ALFBAR,BET
COMMON /METHOD/ STRESS(4),STRAIN(4),D(4,4),IPT,NEL,IPS
EQUIVALENCE (NPAR(5),ITYP2D)
INTEGER PPOT,TMM
SH=(STRESS(1)+STRESS(2)+STRESS(4))/3.
SX=STRESS(1)-SH
SY=STRESS(2)-SH
SZ=SH
SS=STRESS(3)
SM1=SM*3./2.
SM2={ (STRESS(1)-STRESS(2))/2.}*2)+SS**2
PRST1=SM1+DSQRT(SH2)
PRST2=SM1-DSQRT(SH2)
PRST1=PRST1-SH
PRST2=PRST2-SH
DELM=(DELP(1)+DELP(2))/2.
DELM1=((DELP(1)-DELP(2))/2.)*2)+DELP(3)**2
DELT1=DELM+DSQRT(DELM1)
DELT2=DELM-DSQRT(DELM1)
BET=.5*(SX**2+SY**2+SZ**2)+SS**2
PRST1=2*DSQRT(BET)
IF (TMM.EQ.1) ALFT=0.
IF (TMM.EQ.1) ALC=0.
*... THE DP MATRIX-PLANE STRESS *...
*...
IF (IPELD.EQ.3) ALFT=ALC
IF (TMM.EQ.1) GO TO 1
IF (TMM.EQ.1) GO TO 2
RI=(YP*(YKAP-.5*BET)+XF*(YC-SH))
RII=(RII**2)-4*XF*YF*(YKAP*(YC-SH)-YC*BET*0.5)
IF (RII.LT.0) GO TO 3
RI=(DSQRT(RII)-RI)/(2*YF*XF)
GO TO 3
RI=(.5*BET-YKAP)/XF
GO TO 4
IF (RII.LE.0) RI=1E-03
ALFT=(YKAP+RI*XF)/(YC+RI*YF)*3.
IF (RI.LT.0) RI=1E-03
DG=(DELT1*(ALFT+(PRS1/BET)))+(DELT2*(ALFT+(PRS2/BET)))

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YCX04590  
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YCX05450  
YCX05460  
YCX05470  
YCX05480  
YCX05490  
YCX05500

```

G=BET/(3*(PRS1*DELT1+PRS2*DELT2))
IF (PPOT.EQ.2) GO TO 5
G=G*2*(ALFT+(PRS1/BET))*PRST1+(ALFT+(PRS2/BET))*PRST2
G=G/BET
IF (IPELD.EQ.3) BET=ET
IF (IPELD.EQ.3) GO TO 6
IF (RI.LE.RLIM) SET=YH+SHL*RI
IF (RI.LE.RLIM) GO TO 6
EET=EXPA*(2.71828183)**(EXPB*RI)
A2=YH*EET/(YH-EET)
G=G*DG
IF (PPOT.EQ.1) ALFBAR=ALFT
IF (PPOT.EQ.2) ALFBAR=0.
ARA=ALFT*(SX/BET)
ARB=ALFT*(SY/BET)
ARC=2*SS/BET
ARA1=ALFBAR*(SX/BET)
ARB1=ALFBAR*(SY/BET)
BAT=A2*G+A1*((ARA+PV*ARB)*ARA1+(PV*ARA+ARB)*ARB1+B2*(ARC**2))
RLAMBDD=BAT
BAT=A1*A1/BAT
WP=SY*DEPS(1)+SY*DEPS(2)+SS*DEPS(3)
IF (WP.LT.0.) BAT=0.
D(1,2)=A1-(BAT*(ARA1+PV*ARB1)*ARA+(ARA1+PV*ARB1)*PV*ARB)
D(1,3)=B1-(BAT*(ARA1+PV*ARB1)*ARA+(ARA1+PV*ARB1)*PV*ARB)
D(1,4)=B1-(BAT*(ARA1+PV*ARB1)*B2)
D(2,1)=0.
D(2,2)=A1-(BAT*(ARA*(PV*ARA1+ARB1)+PV*ARB*(PV*ARA1+ARB1)))
D(2,3)=A1-(BAT*(PV*ARA*(PV*ARA1+ARB1)+ARB*(PV*ARA1+ARB1)))
D(2,4)=0.
D(3,1)=0.
D(3,2)=0.
D(3,3)=0.
D(3,4)=0.
D(4,1)=0.
D(4,2)=0.
D(4,3)=0.
D(4,4)=0.
.. REDUCE THE STIFFNESS IF THE ELEMENT HAS FAILED ..
.. DO 10 I=1,4
.. DO 10 J=1,4
IF (DABS(D(I,J)).LT.1E-13) D(I,J)=0.
IF (WP.LE.0.) GO TO 11
IF (PPOT.EQ.1) GO TO 12
STRAIN(4)=STRAIN(4)-(DEPS(1)+DEPS(2))
STRAIN(4)=STRAIN(4)+((DELT1+DELT2)/BH)
RETURN
RLMDD=(ARA+ARB*PV)*DEPS(1)+(ARA*PV+ARB)*DEPS(2)+A1*ARC*C1*DEPS(3)
RLMDD=RLMDD/RLAMBDD

```

5 6 C C C C 8 10 12

1 3 5 7 9 11 13 15 17 19 21 23 25 27 29 31 33 35 37 39 41 43 45 47 49 51 53 55 57 59 61 63

YCX05520  
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YCX05540  
YCX05550  
YCX05560  
YCX05570

SPL4=RLABD\*(ALFT+(SZ/BET))  
STRAIN(4)=STRAIN(4)+SPL4\*(DEPS(1)+DEPS(2))\*D1  
RETURN  
STRAIN(4)=STRAIN(4)+D1\*(DEPS(1)+DEPS(2))  
RETURN  
END

11

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## APPENDIX 4

### PROPERTIES RELATING TO THE SHEAVE

#### A4.1 BEAM ELEMENT DIMENSIONS EMPLOYED IN THE PRELIMINARY ANALYSIS OF THE SHEAVE (Chapter 3)

The dimensions selected for representing the rim and spoke sections are illustrated in Fig A4.1. Owing to the naturally rectangular properties of the spoke, the dimensions represent the actual section. However the dimensions selected for the machined rim section (see Section A4.3) are based on representing the depth and second moment of area of the actual section. This approach was chosen since the significant stresses are induced by bending effects. The second moment of area and cross-sectional area of the above mentioned section is tabulated in Table A4.1.

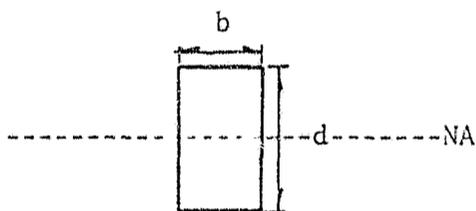


Figure 4.1 - Dimensions of the beam elements

TABLE A4.1 - DIMENSIONS OF THE BEAM ELEMENTS (RIM AND SPOKE)

	b	d	$I_{NA} \text{ (mm}^3\text{)}$
Spoke	31	68	$812,2 \times 10^3$
Rim	37	48	$340 \times 10^3$

#### A4.2 DIMENSIONS OF THE UNMACHINED SHEAVE

The dimensions of the unmachined sheave as used in the previous experiment conducted in 1981 are presented in Fig A4.2

#### A4.3 DIMENSIONS OF THE MACHINED SHEAVE

The dimensions of the machined sheave as employed in the present experiment are illustrated in Fig A4.3.

#### A4.4 CALCULATION OF THE SECOND MOMENT OF AREA OF THE RIM SECTION

The second moment of area, centroid and cross-sectional area were approximated by using the dimensions measured from the actual section. Owing to geometrical imperfections of the rim section introduced during the casting process, the internal and external radii of the section could not be measured accurately. This necessitated the simplification illustrated in Fig A4.4. Hence the calculation of the geometric properties was based on this section, which was divided into seven elements. These calculations are presented in Tables A4.2a and A4.2b, below Fig A4.4.

#### A4.5 GEOMETRIC PROPERTIES OF THE RIM SECTION EMPLOYED IN THE FINITE ELEMENT SIMULATION

As discussed in Chapter 5, the rim section was divided into eight elements, as illustrated in Fig A4.5. The dimensions of each element were selected so that the overall section would approximate the properties calculated in Section A4.4. These dimensions are illustrated in Fig A4.5. The calculation of the second moment of

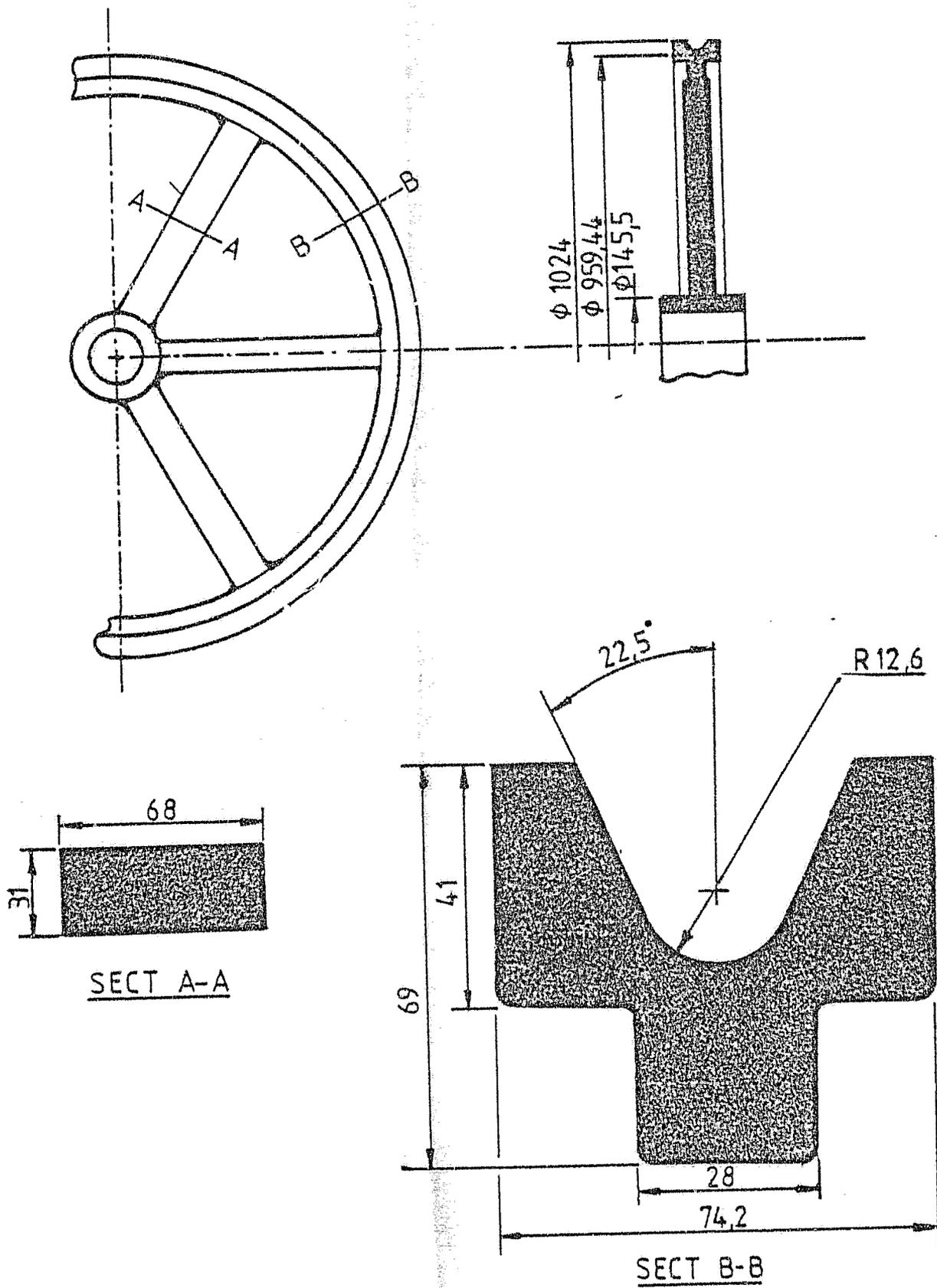


Figure A4.2 - Dimensions of the Unmachined Sheave

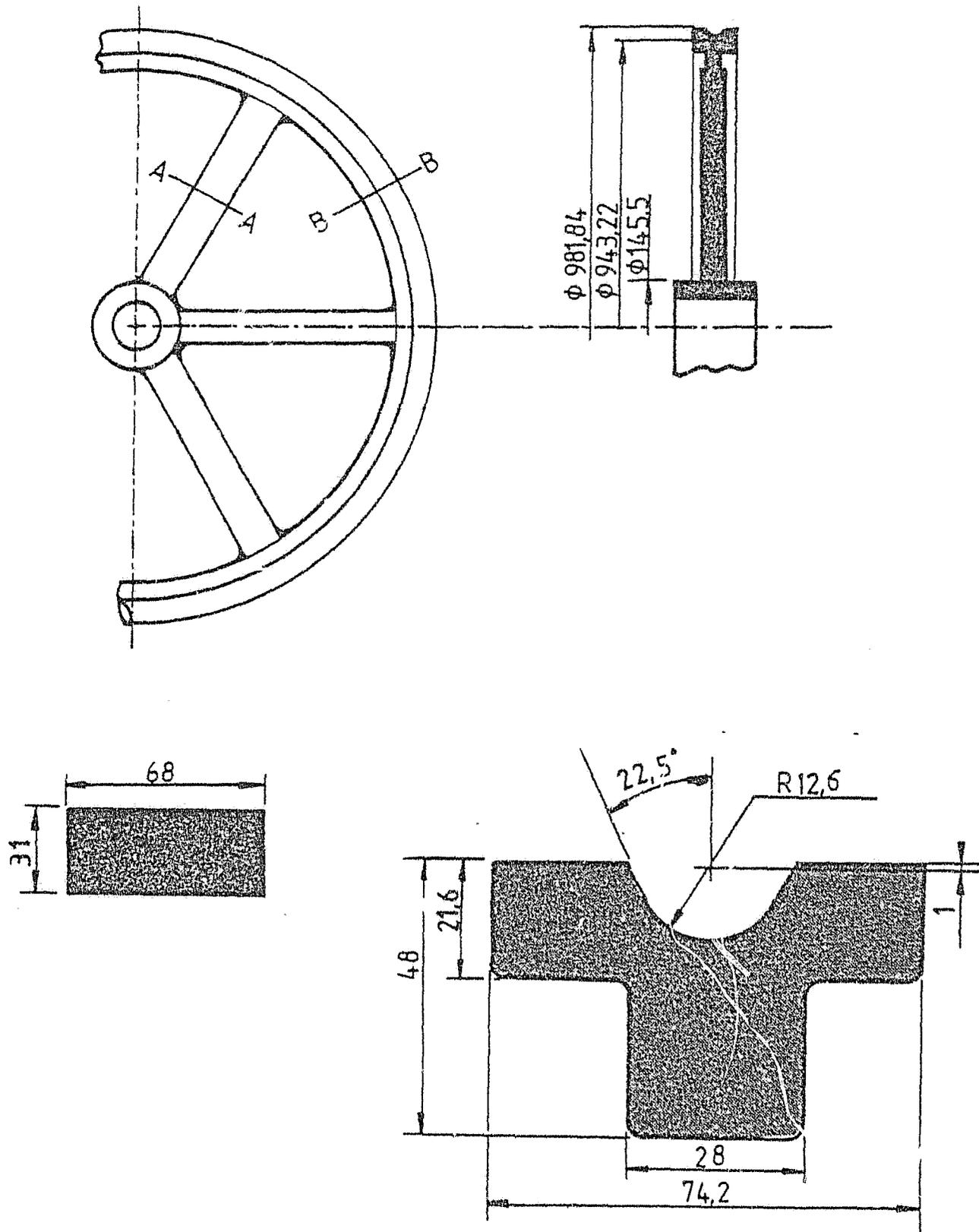


Figure A4.3 - Dimensions of the Machined Sheave

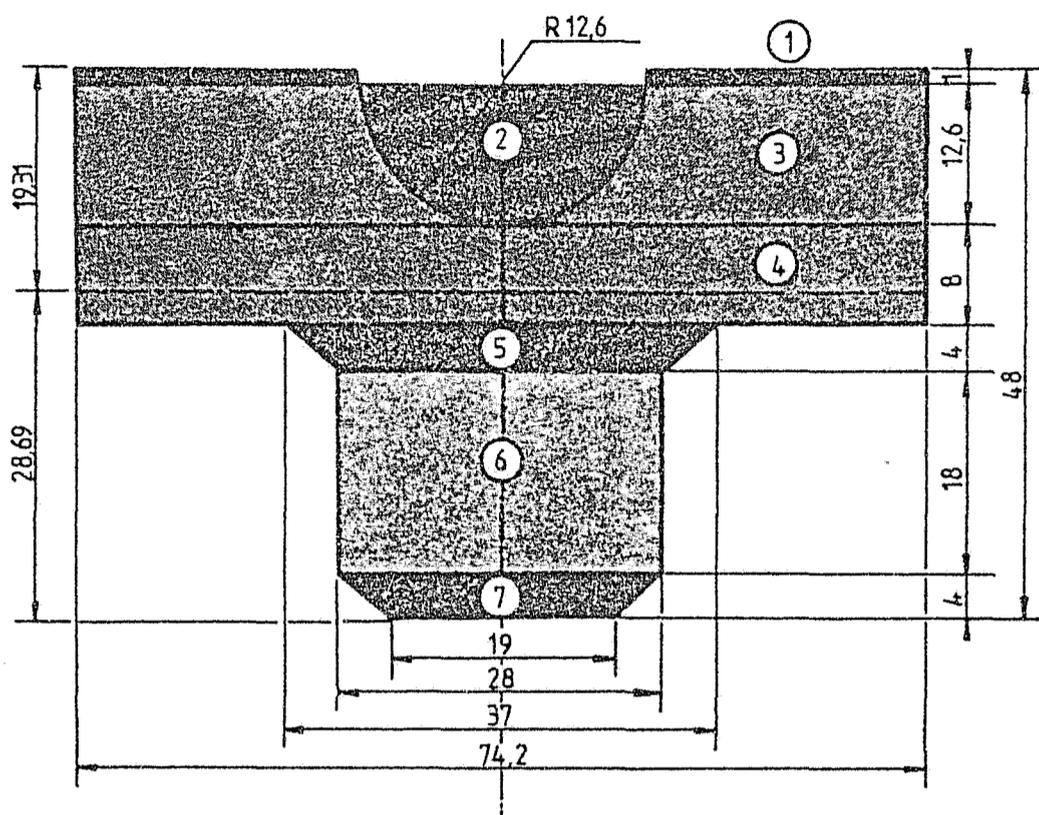


Figure A4.4 - Idealised Cross Section of the Rim

TABLE A4.2a - CALCULATION OF THE CENTROID OF THE SECTION

Element	$A_e$ (mm <sup>2</sup> )	$\bar{y}_e$ (mm)	$\bar{A}\bar{y}_e$ ( $\times 10^3$ )
1	48,00	47,50	2,280
2	-249,37	41,75	-10,411
3	934,92	40,71	38,051
4	584,80	32,42	18,959
5	130,00	24,49	3,184
6	504,00	13,40	6,753
7	105,60	2,32	0,245
	2 057,45		59,060

$$\bar{y} = \frac{\sum A_e \bar{y}_e}{\sum A_e} = 28,69 \text{ mm}$$

TABLE A4.2b - CALCULATION OF THE MOMENT OF INERTIA

Element	$\bar{I}_e$ ( $\times 10^3 \text{mm}^4$ )	d (mm)	$I_{NA}$ ( $\times 10^3 \text{mm}^4$ )
1	2 10 <sup>-</sup>	18,801	16,968
2	-2,767	13,051	-45,241
3	12,368	12,011	147,243
4	3,118	3,721	11,215
5	0,172	- 4,208	2,473
6	13,608	-15,298	131,558
7	0,168	-26,376	73,633
			337,851

$$\bar{I}_{NA} \approx 337,851 \times 10^3 \text{mm}^4$$

area, cross-sectional area and centroid area presented in Table A4.3, below Fig A4.5. It is evident that the second moment of area, centroid and cross-sectional area, compare well with those of the idealised section (Fig A4.5); the discrepancies are - 0,79%, 0,80% and 3,2% respectively. The latter discrepancy of modelling the cross-sectional area is not considered significant since the major effects are due to bending.

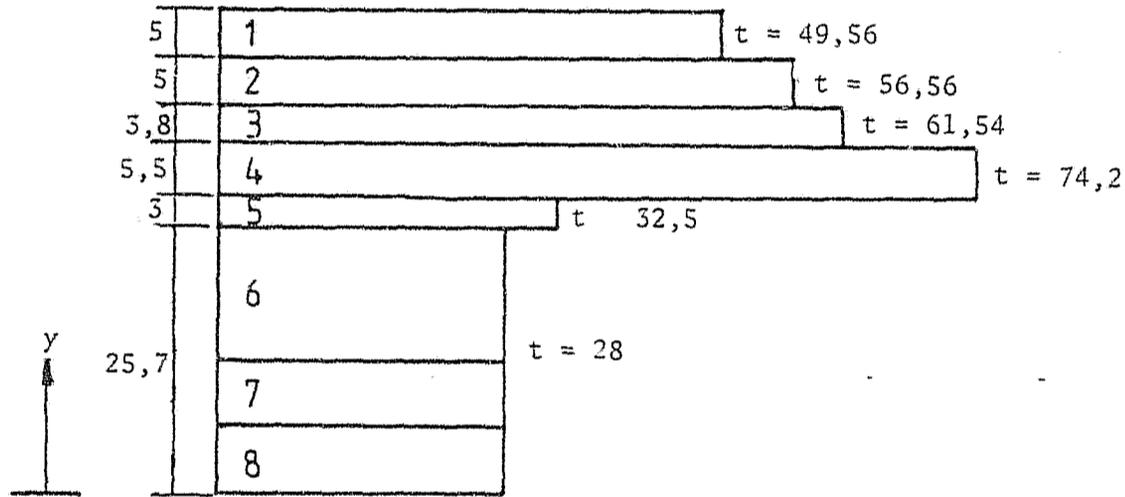


Figure A4.5 - Finite Element Simulation of the Rim Section

TABLE 4.3 - CENTROID, MOMENT OF INERTIA OF SIMULATED SECTION (FEM)

Element	A	$\bar{y}$	$A\bar{y}$	$\bar{I}$	d	$I_{NA}$
1	247,80	45,50	11 274,90	516,25	17,50	76 405,00
2	282,80	40,50	12 160,40	589,16	11,54	37 708,50
3	233,85	36,10	8 442,05	281,40	12,50	42 552,08
4	408,10	31,45	12 834,74	1 028,70	3,45	5 886,16
5	97,50	27,20	2 652,0	73,12	- 0,80	135,52
6,7,8	719,60	12,85	9 246,8	39 607,30	-15,15	204 771,70
	1 989,65		56 610,93			340 531,43

$\bar{y} = 28,452 7 \text{ mm}$

$\bar{I}_{NA} = 340 531,43 \text{ mm}^4$

#### A4.6 METHOD OF CONVERTING THE FINITE ELEMENT RESULTS

As discussed in Chapter 5, the strains predicted by the finite element coordinate analysis are output in terms of the global coordinate axes. The Mohr circle of strain was employed to convert these values to their local coordinate axes for direct comparison with the experimental results.

Fig A4.6 indicates the position of the gauges relative to the integration points and their rotation from the global y-axis. Formula A4.1 was employed to convert the  $\epsilon_y$ ,  $\epsilon_z$ ,  $\gamma_{yz}$  strains into into a reading tangential to the surface of the sheave

$$\epsilon = \frac{\epsilon_y + \epsilon_z}{2} + \frac{\epsilon_y - \epsilon_z}{2} \cos 2\theta + \frac{\gamma_{yz}}{2} \sin 2\theta \quad (\text{gauges 1-15}) \quad (\text{A4.1})$$

$$\epsilon = \frac{\epsilon_y + \epsilon_z}{2} + \frac{\epsilon_y - \epsilon_z}{2} (\cos 2(\theta + 90)) + \frac{\gamma_{yz}}{2} \sin 2(\theta + 90) \quad (\text{gauges 20-23})$$

In some cases the gauge was located between two elements and the reading was taken as the average of the strains of the relevant integration points. Table A4.4 tabulates the relationship between the gauges and the integration points employed in the calculation of the finite element values.

Tables A4.5a and A4.5b tabulate the converted readings from the above mentioned integration points for associated and non associated plastic simulations. These results formed the basis of the values used in Chapter 5.

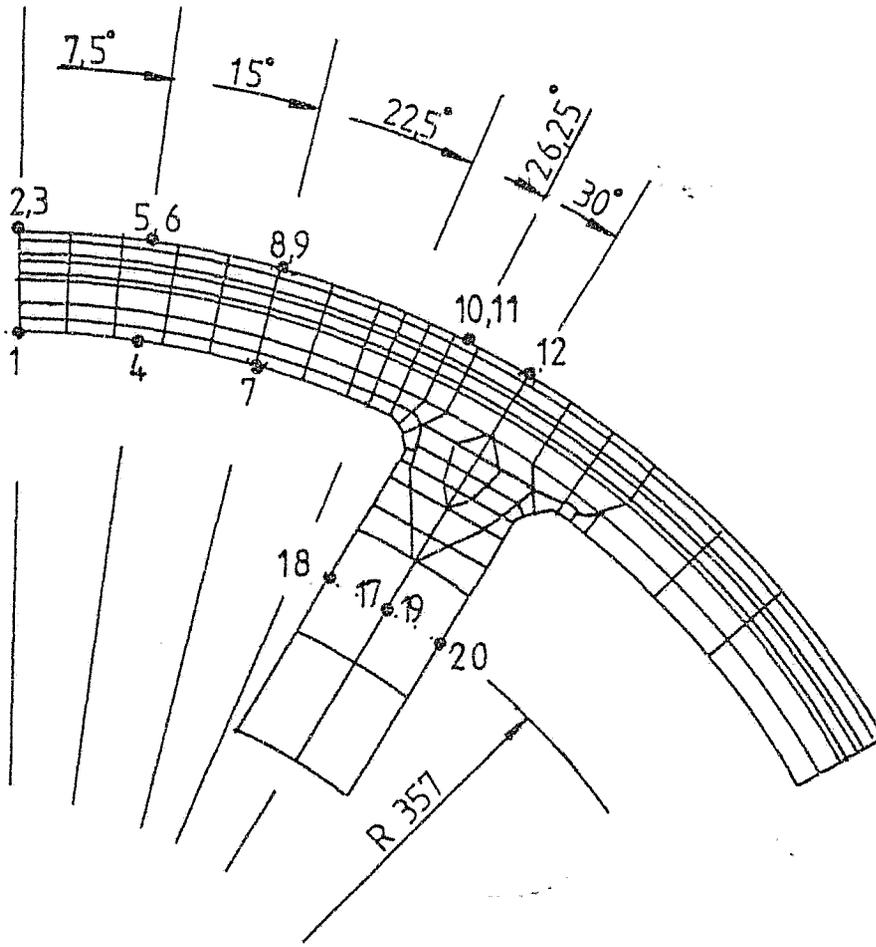


Figure A4.6 - Geometric Location of the Strain Gauges

TABLE A4.4 - ELEMENT/INTEGRATION POINT CORRESPONDING TO THE GAUGE LOCATION

Gauge	Element/Integration Point
1	104-7
2,3	16-9
4	106-4
5,6	16-6
7	109-7/108-1
8,9	11-9/12 - 3
10,11	6-6
12	4-9/5-3
18	134-7
17,19	137-8/134-2
20	137-2

TABLE A4.5a - Associated Plasticity

ROPE TENSION [KN]	ELEMENT/INTEGRATION NUMBER (DEG)							
	104.7 ( 0 )	10.0 ( 0 )	106.4 ( 7.5 )	14.8 ( 7.5 )	109.7 ( 15 )	108.1 ( 15 )	11.0 ( 15 )	12.9 ( 15 )
81.803	718.69	-578.1	551.18	-483.31	14.771	81.751	-120.8	-171.51
123.81	1515	-1178.8	1181.1	-832.98	26.975	180.41	-241.13	-342.24
185.41	2571	-1973.8	1778.4	-1428.1	29.989	229.88	-382.75	-513.94
247.21	4118.1	-2800.2	2589.5	-1993.8	22.734	283.24	-485.8	-892.44
309.02	8388.4	-4137.3	3848.8	-2704.7	-3.0949	313.23	-850.85	-887.32
370.82	8750.8	-8125	4891.3	-3810.5	-80.123	312.18	-892.2	-1095.1
492.82	19889	-8735.9	8788.2	-4817.2	-140.37	284.68	-1048.8	-1328

ROPE TENSION [KN]	ELEMENT/INTEGRATION NUMBER (DEG)						
	8.8 ( 28.25 )	4.8 ( 30 )	5.3 ( 30 )	197.2 ( 30 )	197.8 ( 30 )	194.2 ( 30 )	194.7 ( 30 )
81.803	478.5	300.78	341.85	-23.883	-200.47	-250.58	-378.38
123.81	988.24	804.7	888.15	-47.372	-400.12	-500.27	-755.8
185.41	1834.4	911.75	1081.3	-88.07	-588.52	-748.65	-1127.9
247.21	2553.3	1235.7	1482.3	-78.829	-780.45	-892.78	-1480.9
309.02	3828.8	1578.5	1882.5	-71.348	-892.07	-1255.5	-1870
370.82	5874.8	1925	2538.3	-42.797	-1208.8	-1548.8	-2307.8
492.82	8951.3	2280.3	3187.8	-9.5024	-1452.1	-1873.8	-2704.8

TABLE A4.5b - Non Associated Plasticity

ROPE TENSION (KND)	ELEMENT/INTEGRATION NUMBER (DEG)							
	104.7 ( 0 )	16.9 ( 0 )	108.4 ( 7.5 )	14.6 ( 7.5 )	109.7 ( 15 )	109.1 ( 15 )	11.9 ( 15 )	12.3 ( 15 )
81.809	718.84	-578.1	551.18	-483.91	14.779	81.754	-120.8	-171.52
123.81	1512.5	-1178.9	1117.8	-892.17	28.972	180.41	-241.07	-342.18
185.41	2548.5	-1887.1	1778.2	-1427.8	31.285	231.81	-383.48	-514.17
247.21	4048.8	-2785.8	2588.7	-1985.8	22.528	283.58	-483.42	-680.71
309.02	6308.8	-4131.3	3897	-2707.8	.91096	318.18	-648.8	-884.97
370.82	8518.3	-6088.8	5111.5	-3815.8	-50.401	323.45	-817.8	-1084.8
432.82	13883	-8757.2	8872.2	-4788.8	-128.4	288	-1013	-1300.8

ROPE TENSION (KND)	ELEMENT/INTEGRATION NUMBER (DEG)						
	8.8 ( 28.25 )	4.8 ( 30 )	5.9 ( 30 )	197.2 ( 30 )	197.8 ( 30 )	194.2 ( 30 )	194.7 ( 30 )
81.809	478.52	900.78	941.85	-23.883	-200.47	-250.58	-978.98
123.81	988.88	804.55	888.01	-47.977	-400.14	-500.91	-755.85
185.41	1818.7	812.42	1058.7	-88.8	-588.85	-748.82	-1127.8
247.21	2483.8	1242.2	1480.5	-78.142	-782.95	-895.24	-1482.8
309.02	3785.8	1582.4	1874.8	-70.747	-885.59	-1258.8	-1874
370.82	5552.8	1952.2	2552.4	-43.802	-1215.1	-1551.8	-2281.7
432.82	7810.8	2315.2	3231.5	-3.8439	-1481	-1883	-2708.8

## APPENDIX 5

### ADINA/ADINAT/ADINA-PLOT PROGRAMME EXECUTION

#### A5.1 PROGRAMME EXECUTION ON THE IBM 370

The procedure involved in the preparation and execution of the ADINA/ADINAT/ADINA-PLOT package is illustrated in the flow chart presented in Fig A5.1 below. The VMS/CMS commands employed in this procedure are presented in Table A5.1. The function of the ENGPAC, FADINA, ADINA 81, ADINAT, ADPLOT, will be discussed in the following section.

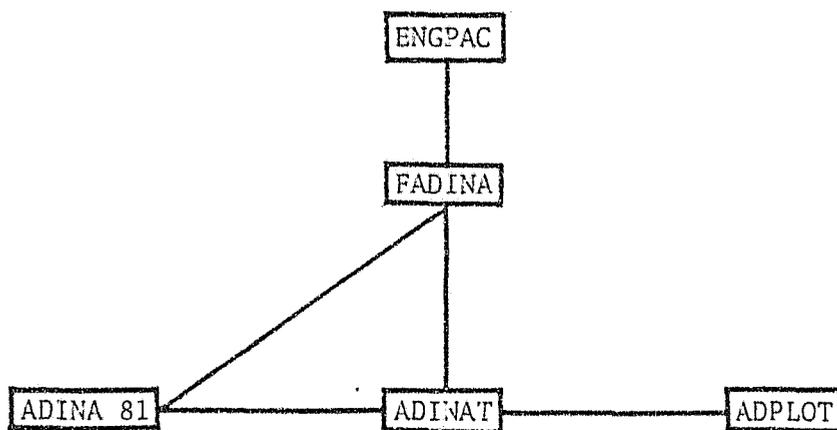


Figure A5.1 - Flow Chart Illustrating ADINA/ADINAT/ADINA-PLOT Execution Procedure

COMMAND	VMS/CMS RESPONSE
DEF STOR 2M  I CMS	ENT STORAGE = 2M R = -----:  ENT CMS
ENGPAC  FADINA 20 <sup>+</sup>	ENT  FORMAT WILL ERASE ALL FILES ON DISK 'G(19F)'. DO YOU WISH TO CONTINUE)? (YES/NO). DMS FOR 605R ENTER DISK LABEL DMS FOR 733I FORMATTING DISK G.
ADINA PROGRAM: ADINA 81 <u>FN</u> <u>FT</u> <u>FM</u> * <u>FN</u> <u>FT</u> <u>FM</u>	ENT INPUT FILE? ENT OUTPUT FILE? EXECUTION BEGINS -----
ADINAT PROGRAM: ADINAT <u>FN</u> <u>FT</u> <u>FM</u> <u>FN</u> <u>FT</u> <u>FM</u>	ENT INPUT FILE? OUTPUT FILE? EXECUTION BEGINS -----
ADINA-PLOT PROGRAM: ADPLOT FN FT FM FN FT FM	ENT INPUT FILE? ENT OUTPUT FILE? ENT EXECUTION BEGINS -----

<sup>+</sup> Number of temporary storage cylinder specified by the user

\* FILE NAME FILE TYPE FILE MODE , eg INPUT DATA A1.

## A5.2 BACKUP ROUTINES

The routines ENGPAC, FADINA, ADINA 81, ADINAT, ADPLOT, were implemented by W Leong of the computer science center. The purpose of these routines is summarised briefly below:

### A5.2.1 ENGPAC<sup>+</sup>

This programme links the user to the disk containing the ADINA/ADINAT/ADINA-PLOT program texts, enabling these programs to be accessed for later use.

### A5.2.2 FADINA<sup>+</sup>

This program creates temporary storage space and defines the logical read write units and the formatting options as required by the program for successful execution. These units provide the slow speed storage as required during execution.

### A5.2.3 ADINA81/ADINAT/ADPLOT<sup>+</sup>

These routines load the respective programs for execution. They direct the input and output listing file as specified by the user.

## A5.3 DEFINING TEMPORARY STORAGE SPACE ON THE READ/WRITE DISK FOR LARGE LISTING FILES

Analyses resulting in large output listing files require the definition of extra storage space if the user's available permanent storage allocation is not to be exceeded ( $\pm 4\ 000$  lines). The VMS/CMS commands

---

<sup>+</sup>The listings of the ENGPAC, FADINA, ADINA81, ADINAT, ADPLOT programs are included at the end of the appendix.

required to define such temporary storage are listed below.

```
DEF □ T3350 □ 195 □ 5          ENT
FORMAT □ 195 □ D              ENT
ACC □ 191 □ C                 ENT
ACC □ 195 □ A                 ENT
```

Copy files from C disk to temporary A disk, ie

```
COPY □ FN □ FT □ FM □ FN □ FT □ A      ENT
```

eg, Copy SHG DATA C SHG DATA A

After execution is complete and the listing file has been printed and sufficient plots obtained, all updated files must be copied to the C disk before signing off.

#### A5.4 REMOTE JOB EXECUTION

In cases where execution may take an extended period of time an option enabling remote execution is available allowing the user to return at a later time when execution is complete.

The commands required to utilise this facility are:

```
#CP SET RUN ON
#CP DISC
```

On issuing these commands the user has effectively terminated the session on the terminal without affecting program execution. The terminal may then be utilised by another user.

On reconnecting, the following command must be issued:

B

which notifies the computer that he is once again connected as an ordinary user.

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ENGPAC - listing

FILE: ENGPAC EXEC Y2

STRACE ERR  
CP LINK ENG 196 19B RR STATS  
ACCESS 19B B/A



ADINA81 - listing

```
FILE: ADINA81 EXEC B1
&TRACE OFF
&TYPE VARS &ENTER DATA FILE ID:
&READ VARS &INNAME &INTYPE &INMODE
&TYPE VARS &ENTER OUTPUT FILE ID:
&READ VARS &OUTNAME &OUTTYPE &OUTMODE
GLOBAL, TXTLIB CMSLIB FORTYLIB
LOAD ADINA81
FY 5 DISK &INNAME &INTYPE &INMODE
FY 6 DISK &OUTNAME &OUTTYPE &OUTMODE
START
&EXIT
```

ADPLOT - listing

```

1  FILE: ADPLOT EXEC B1
3
5  ETRACE ERR ENTER DATA FILE ID:
7  ETYPE VARS EINNAME EINTYPE EINMODE
9  EREAD VARS ENTER OUTPUT FILE ID:
11 EAGAIN VARS EOUTNAME EOUTTYPE EOUTMODE
13 EREAD VARS ENTER "N" OR "W" (N=NARROW PAPER (3DCM), W=WIDE PAPER (83CH)) :
15 EIF .EPAPER EQ .N EGOTO -PAPH
17 EIF .EPAPER EQ .W EGOTO -DAPH
19 ETYPE : EPAPER : IS AN INVALID ARGUMENT.
21 EGOTO -AGAIN
23 -PAPH EWIDTH = P
25 -DAPH EWIDTH = Q
27 -LIN CP LINK GRAPHICS 191 199 RR READ
29 AC 199 H
31 STACKID EINNAME
33 X @ $ TLIB (PROF @@@
35 GL TLIB CMSLIB FORTLIB GRAPHICS
37 LOAD ADPLOT
39 ERCC = ERC
41 EIF ERC NE 0 EGOTO -FAIL
43 FI FT01 001 CLEAR
45 FI FT05F001 CLEAR
47 FI FT06F001 CLEAR
49 FI FT18F001 CLEAR
51 FI FT19F001 CLEAR
53 FI FT50F001 CLEAR
55 FI FT60P001 CLEAR
57 FI 1 DISK DATA BASE G (RECFM F LRECL 2000 BLOCK 2000 DSORG DA XTENT 790
59 FI 5 DISK EINNAME EINTYPE EINMODE
61 FI 6 DISK EOUTNAME EOUTTYPE EOUTMODE
63 FI 18 DISK @ $$ G
65 FI 19 DISK @ $$ G
67 FI 50 DISK EINNAME PLOT G (RECFM VS BLKSIZE 368
69 FI 60 DISK PLOT DATA G (RECFM VBS BLOCK 4000
71 START = ERC
73 ERCC = ERC
75 EIF ERC NE 0 EGOTO -FAIL
77 CP SP PU CALPLOT NOH HOC CL G CO 1 D EWIDTH
79 DISK DUMP EINNAME PLOT G
81 CP SP PU OFF CL A D OFF
83 -FAIL FI * CLEAR
85 GL TLIB
87 REL H (DET
89 ERCC
91
93
95
97
99
101
103

```

ADPLOT - listing

```

1 FILE: ADPLOT EXEC B1
3
5 TRACE ERR ENTER DATA FILE ID:
7 STYPE VARS EINNAME EINTYPE EINMODE
9 STYPE VARS ENTER OUTPUT FILE ID:
11 STYPE VARS EOUTNAME EOUTTYPE EOUTMODE
13 -AGAIN STYPE ENTER "N" OR "W" (N=NARROW PAPER (30CM), W=WIDE PAPER (83CM)) :
15 GIFF .GPAPER EQ .N EGOTO -PAPH
17 GIFF .EPAPER EQ .W EGOTO -DAPH
19 STYPE ; GPAPER ; IS AN INVALID ARGUMENT.
21 EGOTO -AGAIN = P
23 -PAPH EWIDTH = Q
25 -EGOTO -LIN
27 -PAPH CP LINK GRAPHICS 191 199 RR READ
29 -AC 199 H
31 STACKID EINNAME
33 X @ $ (PROP @@@
35 GL TXLLIB CMSLIB FORTXLIB GRAPHICS
37 LOAD ADPLOT
39 ERCC = ERC
41 EIF ERC NE 0 EGOTO -FAIL
43 FI FT01F001 CLEAR
45 FI FT05F001 CLEAR
47 FI FT06F001 CLEAR
49 FI FT18F001 CLEAR
51 FI FT19F001 CLEAR
53 FI FT50F001 CLEAR
55 FI FT60F001 CLEAR
57 FI 1 DISK DATA BASE G (RECFM F LRECL 2000 BLOCK 2000 DSORG DA XTENT 790
59 FI 5 DISK EINNAME EINTYPE EINMODE
61 FI 6 DISK EOUTNAME EOUTTYPE EOUTMODE
63 FI 18 DISK @ $$ G
65 FI 19 DISK @ $$ G
67 FI 50 DISK EINNAME PLOT G (RECFM VBS BLKSIZE 368
69 FI 60 DISK PLOT DATA G (RECFM VBS BLOCK 4000
71 START = ERC
73 ERCC = ERC
75 EIF ERC NE 0 EGOTO -FAIL
77 CP SP PU CALPLOT HON HOC CL G CO 1 D EWIDTH
79 DISK DUMP EINNAME PLOT G
81 CP SP PU OFF CL A D CFF
83 -FAIL FI * CLEAR
85 GL TXLLIB
87 REL H (DET
89 ERXIT &RCC

```

## APPENDIX 6

### UNDERGRADUATE PROJECTS (1982-83) RELEVANT TO THE DEVELOPMENT OF THE FINITE ELEMENT METHOD AT THE UNIVERSITY OF THE WITWATERSRAND

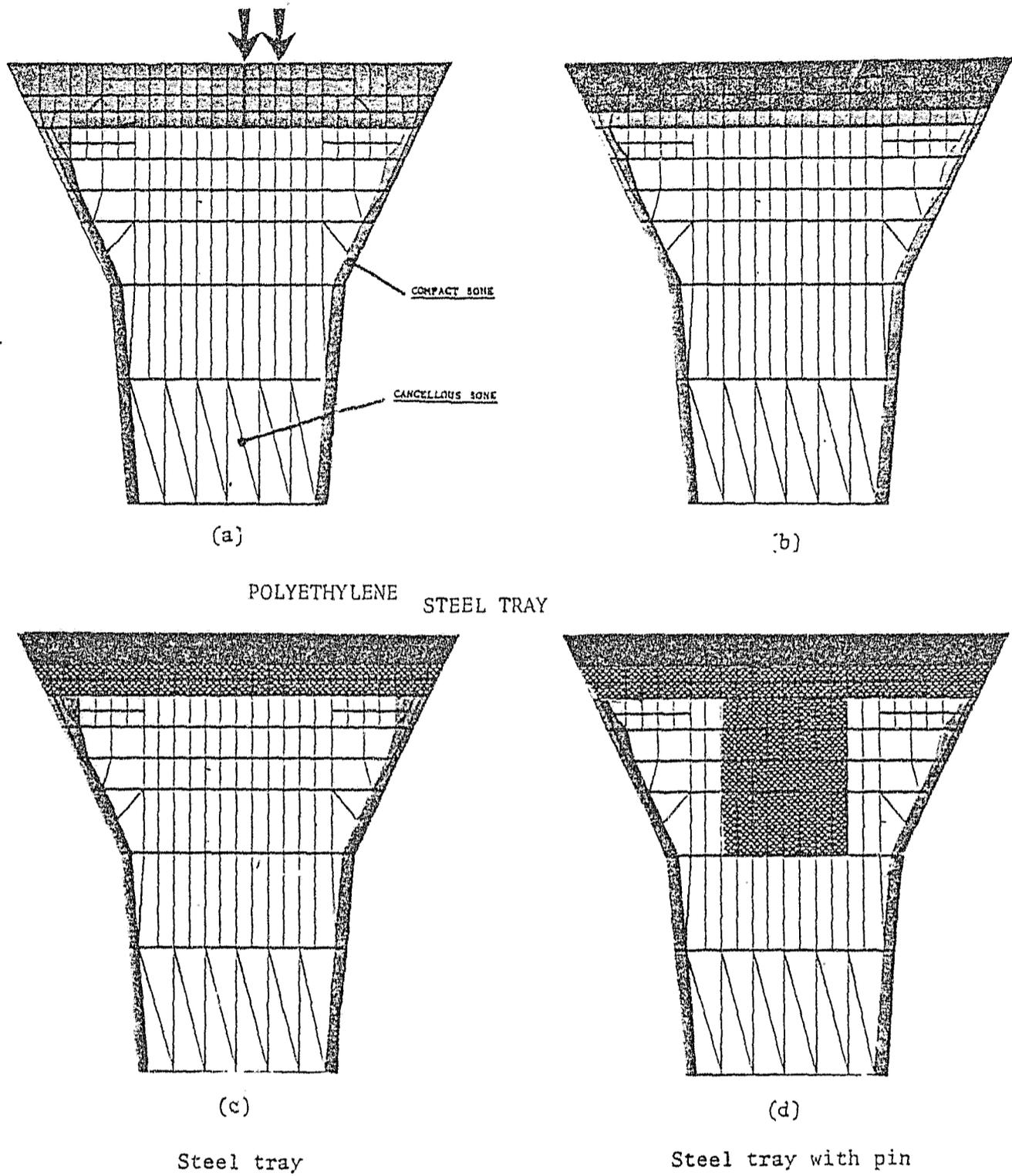
#### A6.1 REPLACING THE HUMAN KNEE JOINT: (G HACK, N LANE - BSc Design Report, Sept 1982. Supervisor, Dr H D Chandler)

This project consists of an overall view of knee prosthesis design with special reference to the van Reenen Knee. In order to assess the van Reenen knee prosthesis with regard to its tibial configuration a comparative study involving a finite element simulation of four different configurations (including that of the van Reenen knee) was undertaken.

The configurations examined were:

- The natural tibia.
- An all polyethylene tibial component (as used in the van Reenen knee).
- A polyethylene bearing surface supported by a metal tray.
- As above but with a pin attached to the lower surface of the metal tray.

The tibia was modelled with plain strain two-dimensional elements. The properties of the compact cancellous bone and the polyethylene/steel tray were accounted for by defining four material property sets applied to the relevant areas of the mesh indicated in Fig 6.1.



- (a) Tibia modelled with compact and cancellous bone only
- (b) Polyethylene tibial component
- (c) Pin attached to the lower surface of the metal tray

Figure 6.1 - Analysis of a Knee Joint Prosthesis

The results of this study indicate that improved performance of the van Reenen knee can be achieved by utilising a polyethelene bearing surface supported by a steel tray. This observation is drawn from a careful examination of the stresses induced in the above configuration which closely resembles those predicted in the analysis of the natural tibia. The writer acted as advisor in the use of ADINA.

A6.2 PREPROCESSOR PROGRAM: (S PAVLOVIC - BSc Design Report, Sept 1982. Supervisors: Prof C Dimitriou & Mr C Constancon

This project involved the preparation of a preprocessor programme capable of interactively structuring an input data file in the format required by the ADINA programme. At present the programme is partially complete, facilitating data preparation up to the beginning of element data input. Further work is required to complete this facility and to extend it to include data preparation for the ADINAT programme.

A6.3 FINITE ELEMENT ANALYSIS OF A THIN SQUARE PLATE:  
(A BIZOS - BSc Minilab Report, July 1983. Supervisors:  
Prof C Dimitriou & Mr C Constancon

This project involved utilising shell/plate shell elements to approximate the central deflection of a thin square plate subjected to a uniformly distributed pressure load normal to its surface. Small deflections were considered and the material was assumed isotropic; ie a linear elastic analysis was performed.

The performance of the two element types was assessed with regard to

- (i) Shell elements:
- numerical integration order
  - four, eight, nine, sixteen noded elements

- mesh refinement
- (ii) Plate Shell elements:
- mesh refinement<sup>1</sup>

The results obtained were compared with a theoretical solution for the purpose of assessing the effect of the constraints listed above on the solution accuracy. In addition a geometrically nonlinear (total langrangian) analysis of the plate provided confirmation that the deflections imposed in the analyses were within the linear region.

The main purpose of this study was to introduce the pertinent aspects of the finite element method through the preparation of input data files, execution of the analyses and the interpretation of the results. This approach illustrated in a practical manner the importance of mesh refinement, element order, numerical integration order and the correct imposition of boundary conditions.

The results of the study confirmed that

- underintegration generally results in higher accuracy
- convergence to the true solution occurs with increasing mesh refinement and higher order elements
- the choice of mesh has to draw on finite element theory and past experience.

A preliminary assessment of the accuracy achieved in approximating the natural frequencies of plates by employing the two types of elements discussed indicated that the solutions were an order of magnitude different for similar meshes. This is not documented in

---

1. Plate shell elements are 3-noded elements which for a linear elastic analysis have a fixed numerical integration order of  $2 \times 2$ .

the report since a fuller investigation is required before conclusive statements can be made. This is likely to be the topic of future study.

## APPENDIX 7

### ADINA-PLOT, PROGRAM DESCRIPTION AND INSTALLATION

#### A7.1 PROGRAM DESCRIPTION

The ADINA-PLOT program constitutes a post-processing facility (ie after employing the ADINA package) enabling rapid interpretation of the results obtained from a finite element simulation. A flow chart illustrating its mode of operation in conjunction with the ADINA program is shown below in Fig A7.1.

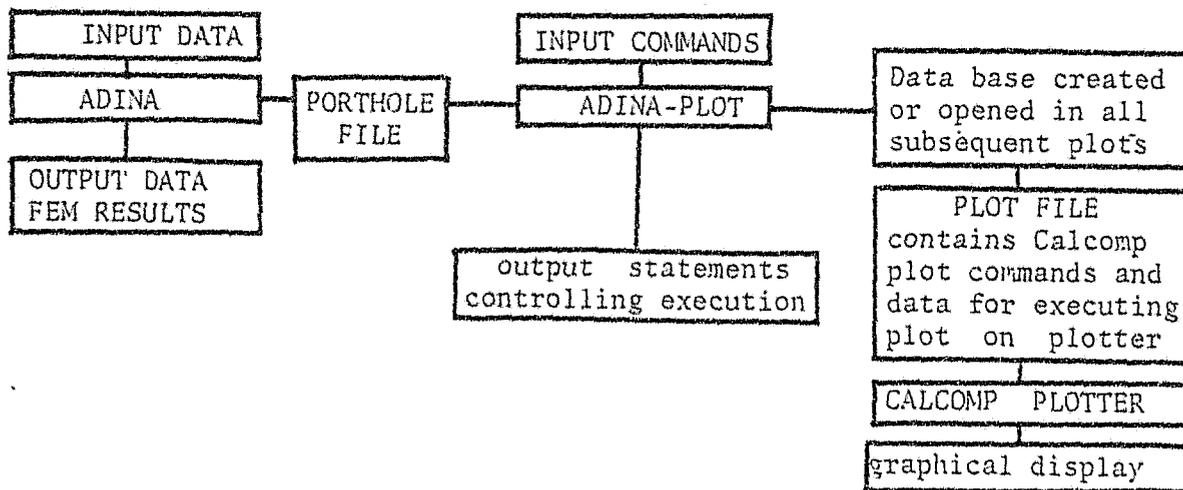


Figure A7.1 - Flow Chart Illustrating the Interaction between the ADINA/ADINA -PLOT PROGRAMS

A brief outline of the mode of operation is presented but a more detailed explanation can be found in the ADINA-PLOT user's manual [9]. The input and output data of a finite element simulation may be channelled by ADINA to a file called the porthole file. ADINA-PLOT reads this generated file in order to load a specifically designed data base which stores the data on a random access file for easy retrieval. Once the data base is loaded with data from an ADINA run it can be

accessed by ADINA-PLOT any number of times, hence the porthole file becomes redundant.

The input command file contains a command language which specifies the format and type of plot required. Default options are available which ensure that a satisfactory plot will be obtained with the minimum amount of input data.

Two types of error recovery are available to decide on a solution strategy when an error is encountered during execution. Either the execution can be terminated or it can be continued at the next command in the input file.

On successful execution, a plot file containing calcomp plotting commands is submitted directly to the calcomp plotter for a graphical output. Simultaneously, the output from the ADINA-PLOT can be directed to three print files: ECHO; LOG and LIST. The ECHO file contains an echo printing of the commands and parameters as they had been defined in the input file. The LOG file contains relevant diagnostic messages generated during the execution, whilst the LIST file contains selective data listings which can be requested by the user.

## A7.2 INSTALLATION

ADINA-PLOT was installed on the IBM 370 in March 1983. The installation involved writing a backup routine ADPLOT which, prior to execution, defined the logical read write units used in the source as well as the location of the input command and output listing files. Ultimately this routine links the ADINA-PLOT program to the calcomp plotting library and directs the plot file created during execution to the calcomp plotter for a graphical output.

The test analysis supplied with the ADINA/ADINA-PLOT source<sup>1</sup> was

---

1. Dynamic analysis of an eight storey building

executed in order to verify that the program was operating correctly. Satisfactory execution was not achieved since the ADINA-PLOT program did not generate the required data base. Further attempts to achieve a satisfactory execution were unsuccessful and outside help was sought [26].

Errors were apparent in both the backup routine FADINA<sup>2</sup> and the source. It was evident that the logical write statement defining the format of the data base file was inconsistent for a random access file. In the original FADINA routine the following statement was used with no formatting options, ie

```
F I 1 DISK DATA G
```

For a random access file this should be defined as

```
F I 1 DISK DATA G (RECFM VBS BLOCK 4000)
```

By defining this logical write statement, the porthole file was created successfully. However in early test program runs it became apparent that the first statement in the input command file was not correctly executed by the program causing premature termination. This was overcome by placing a redundant statement at the beginning of the file and selecting the error recovery option which, upon the discovery of an error, proceeded to the next statement in the input file. Thus for future work, until this problem is rectified, the following statements should always be specified at the beginning of any input command file

```
FILE  
CONTROL B = 0  
.  
.  
.  
END
```

---

2. A listing of the FADINA routine is presented in Appendix 5 together with a brief description.

**Author** Constancon C P

**Name of thesis** A study of the finite element method, with reference to the Adina finite element package 1983

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